

#### **ADDENDUM NO. 1**

To: All Plan Holders of Record and Interested Parties

Project: Clinton County Administration Building - Addition & Alterations

Project No.: 22072

Issue Date: February 16, 2024

Bids Due: 2:00 PM, March 13, 2024

The following additions, revisions, corrections, and clarifications contained herein shall become part of the Construction Contract Documents for the Project and shall be included in the Scope of Work and Bid Proposals to be submitted. References made below to Specifications and Construction Drawings shall be used as a general guide only. Bidders shall determine for themselves the full scope of work affected by the Addendum items.

The following Addendum is part of the Construction Contract Documents dated February 13, 20924.

This Addendum consists of the following:

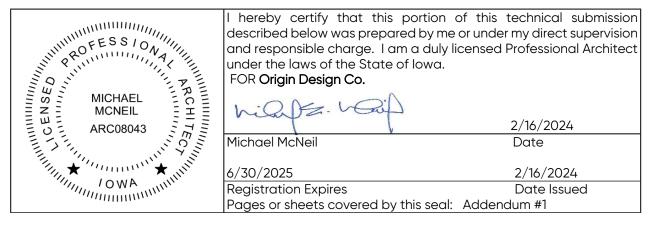
Addendum No. 1 -- 1 Page

6 Plan Sheets - Attached Structural Drawings

#### CHANGES TO DRAWINGS:

- See attached structural plan sheets.
  - a. S0.1 ABREVIATIONS AND STRUCTURAL NOTES
  - b. S1.1 OVERALL FOUNDATION PLAN
  - c. S1.2 VESTIBULE FOUNDATION PLAN AND DETAILS
  - d. S3.1 VESTIBULE ROOF PLAN AND SECTIONS
  - e. S6.1 TYPICAL STEEL DETAILS
  - f. S6.2 DETAILS

It is required to acknowledge this addendum on the Bid Proposal Form.



END OF ADDENDUM NO. 1

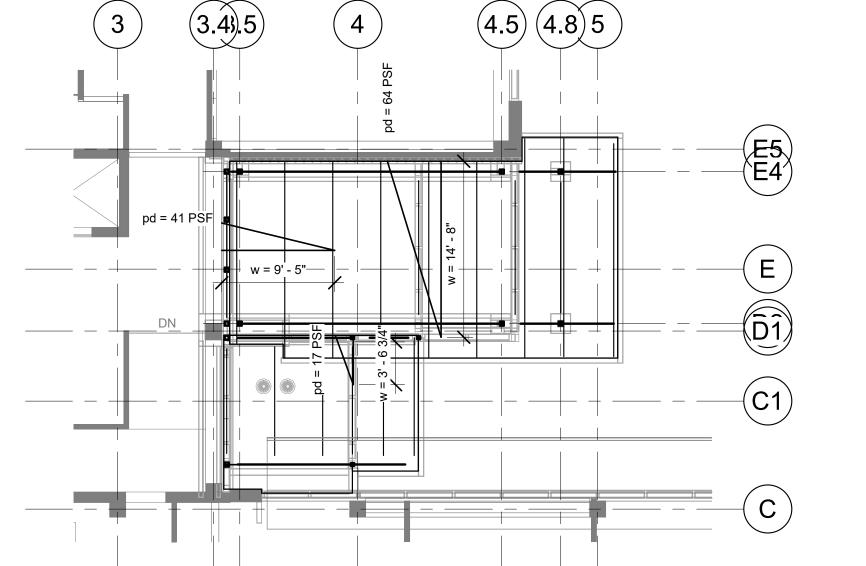
	ANCHOR BOLT	HEF	HORIZONTAL EACH FACE
D'L	ADDITIONAL	HIF	HORIZONTAL LACITY ACE
=	ABOVE FINISHED FLOOR	HK	HOOK
	ALTERNATING	HM	HOLLOW METAL
JM CH	ALUMINUM ARCHITECT/ARCHITECTURAL	HOF HORIZ	HORIZONTAL OUTSIDE FACE HORIZONTAL
оп Э	AVERAGE	HSA	HEADED STUD ANCHOR
	7.02.0.02	HSS	HOLLOW STRUCTURAL SECTION
	BOTTOM OF	HT	HEIGHT
	BOND BEAM		T
VC(C)	BRICK LEDGE	ID I.F.	INSIDE DIAMETER/INSIDE DIMENSION INSIDE FACE
)G(S) Γ	BUILDING(S) BOTTOM	IJ	ISOLATION JOINT
3	BEARING	IMP	INSULATED METAL PANEL
(	BRICK	IN	INCHES
۷N	BETWEEN	INFO	INFORMATION
NT	CANTILEVER	INSP INSUL	INSPECTION INSULATION
N I	CENTER TO CENTER	INT	INTERIOR
	COLD-FORMED		
	CAST-IN-PLACE	JT	JOINT
	CONTROL JOINT		Luna
	CLEAR	K	KIPS PER LINEAL FOOT
J	CLEAR CONCRETE MASONRY UNIT	KLF KIP	KIPS PER LINEAL FOOT  1 KIP = 1,000 LBS
-	CLEAN OUT	KSF	KIPS PER SQUARE FOOT
	COLUMN		
VC	CONCRETE	L	ANGLE
NN IOT	CONNECTION	LB/#	POUND
NST NT	CONSTRUCTION CONTINUOUS	LF	LINEAL FOOT LIVE LOAD
ORD	COORDINATE	LL LLH	LONG LEG HORIZONTAL
R(D)	CENTER, CENTERED	LLV	LONG LEG VERTICAL
₹	CENTER	LONG	LONGITUDINAL
	CUBIC YARD	LW	LIGHT WEIGHT
<del></del>	DEGREE	LVL	LAMINATED VENEER LUMBER
L	DEFLECTION	MAS	MASONRY
<u> </u>	DIAMETER	MAX	MAXIMUM
G	DIAGONAL	MECH	MECHANICAL
	DIMENSION	MEZZ	MEZZANINE
Т	DISTANCE	MFR(S)	MANUFACTURER(S)
0(0)	DEAD LOAD	MIN	MINIMUM
G(S)	DRAWING(S)	MISC MP	MISCELLANEOUS METAL PANEL
	EAST	MS	METAL STUD(S)
	EACH		
	EACH END	N	NORTH
	EACH FACE	N/A	NOT APPLICABLE, NOT AVAILABLE
	EXPANSION JOINT ELEVATION	NF NO or #	NEAR FACE NUMBER
C	ELECTRICAL	N.S.	NEAR SIDE
BED	EMBEDMENT	NIC NIC	NOT IN CONTRACT
₽R	ENGINEER	NOM	NOMINAL
,	EPOXY	NTS	NOT TO SCALE
IID	EQUAL	OC	ON CENTER
JIP	EQUIPMENT EXPOSED STRUCTURE	OD	OUTSIDE DIAMETER
<u> </u>	EXISTING TO REMAIN	0.F.	OUTSIDE FACE
	EXISTING	ОН	OVERHEAD
)	EXPANSION	OPG	OPENING
	EXTERIOR	OPP	OPPOSITE OPP
	EACH WAY	OSB	ORIENTED STRAND BOARD
	FACE OF	PAF	POWDER-ACTUATED FASTENER
	FLOOR DRAIN	PC	PRECAST/PRESTRESSED CONCRETE
	FOUNDATION	PCF	POUNDS PER CUBIC FOOT
	FACE TO FACE	PERF	PERFORATED
	FAR FACE	PERIM	PERIMETER
	FINISH FLOOR ELEVATION FINISHED	PERP P/L	PERPENDICULAR PROPERTY LINE
	FLOWLINE	PL	PLATE
	FLOOR	PLF	POUNDS PER LINEAL FOOT
1	FIBER REINFORCED PLASTIC	PRELIM	PRELIMINARY
	FAR SIDE	PROJ	PROJECTION
	FOOTING	PROP	PROPOSED  POLINDS BER SOLIABE FOOT
i	FOOTING FIELD VERIEY	PSF PSI	POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH
	FIELD VERIFY	PT PSI	PAINT/PAINTED
	GAGE OR GAUGE	PVMT	PAVEMENT
	GENERAL CONTRACTOR		
V	GALVANIZED	QTY	QUANTITY
ILAM	GLUE LAMINATED WOOD	RD	ROOF DRAIN OVERELOOW
<u>)</u>	GRADE GYPSUM	RDO REBAR	ROOF DRAIN OVERFLOOW REINFORCING BAR
	OTT GOIVI	REBAR	REFERENCE

REINF	REINFORCING/REINFORCED
REM	REMAINDER
REQ	REQUIRED
REQ'S	REQUIREMENTS
REV RM	REVISION ROOM
RO	ROUGH OPENING
RP	RADIUS POINT
RTU	ROOFTOP UNIT
	COUTU
S	SOUTH
SB SCH('D)	SOIL BORING SCHEDULE/SCHEDULED
SD	SUB DRAIN
SDL	SUPERIMPOSED DEAD LOAD
SF	SQUARE FOOT
S.F.	SPLIT FACE
SIM	SIMILAR
SLL SOG	SUPERIMPOSED LIVE LOAD SLAB ON GRADE
SP	SPACE
SPEC(S)	SPECIFICATION(S)
SPEC'D	SPECIFIED
SQ	SQUARE
SS	STAINLESS STEEL
STAG	STAGGERED
STD	STANDARD
STIFF STL	STIFFENER STEEL
STR	STRUCTURE/STRUCTURAL
SY	SQUARE YARD
SYM	SYMBOL
T/	TOP OF
TEMP	TERMINATE
TERM	TOD OF CRADE BEAM
T/GRACE BEAM THK	TOP OF GRADE BEAM THICK/THICKNESS
TPG	TOPPING
TRANS	TRANSVERSE
TRTD	TREATED
TS	TUBE STEEL
TYP	TYPICAL
UL	UNDERWRITERS LABORATORIES, INC
UNO	UNLESS NOTED OTHERWISE
VAR	VARIES
VEF	VERTICAL EACH FACE
	VEDTIOA
VERT	VERTICAL INCIDE FACE
VOE	VERTICAL OUTSIDE FACE
VOF	VERTICAL OUTSIDE FACE
W	WEST
W/	WITH
V V /	WITHOUT
W/O WP	WORKING POINT
W/O WP WD	WORKING POINT WOOD
W/O WP WD WT	WORKING POINT WOOD WEIGHT
W/O WP WD WT	WORKING POINT WOOD
W/O WP WD WT WWF	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC
W/O WP WD WT WWF	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC CROSS
W/O WP WD WT WWF	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC CROSS EXPOSED
W/O WP WD WT WWF	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC CROSS
W/O WP WD WT WWF X XP XSTG	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG
W/O WP WD WT WWF X XP XSTG	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG
W/O WP WD WT WWF  X XP XSTG XXSTG	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG
W/O WP WD WT WWF  X XP XSTG XXSTG	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES
W/O WP WD WT WWF  X XP XSTG XXSTG YD  IND	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE
W/O WP WD WT WWF X XP XSTG XXSTG YD IND ACI ADA	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES
W/O WP WD WT WWF  X XP XSTG XXSTG  YD  IND  ACI ADA	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE AMERICANS WITH DISABILITIES ACT
W/O WP WD WT WWF  X XP XSTG XXSTG YD  IND  ACI ADA AISC	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE AMERICANS WITH DISABILITIES ACT AMERICAN INSTITUTE OF STEEL CONSTRUCTION
W/O WP WD WT WWF X XP XSTG XXSTG YD IND ACI ADA AISC APA ASCE	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE AMERICANS WITH DISABILITIES ACT AMERICAN INSTITUTE OF STEEL CONSTRUCTION THE ENGINEERED WOOD ASSOCIATIC AMERICAN SOCIETY OF CIVIL ENGINE
W/O WP WD WT WWF X XP XSTG XXSTG YD IND ACI ADA AISC APA ASCE	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE AMERICANS WITH DISABILITIES ACT AMERICAN INSTITUTE OF STEEL CONSTRUCTION THE ENGINEERED WOOD ASSOCIATIC AMERICAN SOCIETY OF CIVIL ENGINE AMERICAN SOCIETY OF
W/O WP WD WT WWF  X XP XSTG XXSTG  YD  IND  ACI ADA AISC  APA ASCE ASTM	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE AMERICANS WITH DISABILITIES ACT AMERICAN INSTITUTE OF STEEL CONSTRUCTION THE ENGINEERED WOOD ASSOCIATIC AMERICAN SOCIETY OF CIVIL ENGINE AMERICAN SOCIETY OF TESTING AND MATERIALS
W/O WP WD WT WWF X XP XSTG XXSTG YD IND ACI ADA AISC APA ASCE ASTM AWS	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE AMERICANS WITH DISABILITIES ACT AMERICAN INSTITUTE OF STEEL CONSTRUCTION THE ENGINEERED WOOD ASSOCIATIC AMERICAN SOCIETY OF CIVIL ENGINE AMERICAN SOCIETY OF TESTING AND MATERIALS AMERICAN WELDING SOCIETY
W/O WP WD WT WWF X XP XSTG XXSTG YD IND ACI ADA AISC APA ASCE ASTM AWS IBC	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE AMERICANS WITH DISABILITIES ACT AMERICAN INSTITUTE OF STEEL CONSTRUCTION THE ENGINEERED WOOD ASSOCIATIC AMERICAN SOCIETY OF CIVIL ENGINEI AMERICAN SOCIETY OF TESTING AND MATERIALS AMERICAN WELDING SOCIETY INTERNATIONAL BUILDING CODE
W/O WP WD WT WWF X XXP XSTG XXSTG YD IND ACI ADA AISC APA ASCE ASTM AWS IBC ICC	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE AMERICANS WITH DISABILITIES ACT AMERICAN INSTITUTE OF STEEL CONSTRUCTION THE ENGINEERED WOOD ASSOCIATIO AMERICAN SOCIETY OF CIVIL ENGINES AMERICAN SOCIETY OF TESTING AND MATERIALS AMERICAN WELDING SOCIETY INTERNATIONAL BUILDING CODE INTERNATIONAL CODE COUNCIL
W/O WP WD WT WWF X XXP XSTG XXSTG YD IND ACI ADA AISC APA ASCE ASTM AWS IBC ICC IEBC	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE AMERICANS WITH DISABILITIES ACT AMERICAN INSTITUTE OF STEEL CONSTRUCTION THE ENGINEERED WOOD ASSOCIATIO AMERICAN SOCIETY OF CIVIL ENGINES AMERICAN SOCIETY OF TESTING AND MATERIALS AMERICAN WELDING SOCIETY INTERNATIONAL BUILDING CODE INTERNATIONAL CODE COUNCIL INTERNATIONAL EXISTING BUILDING CO
W/O WP WD WT WWF X XP XSTG XXSTG YD IND ACI ADA AISC APA ASCE ASTM	WORKING POINT WOOD WEIGHT WELDED WIRE FABRIC  CROSS EXPOSED EXTRA STRONG DOUBLE EXTRA STRONG  YARD  USTRY STANDARD AND CODES  AMERICAN CONCRETE INSTITUTE AMERICANS WITH DISABILITIES ACT AMERICAN INSTITUTE OF STEEL CONSTRUCTION THE ENGINEERED WOOD ASSOCIATIO AMERICAN SOCIETY OF CIVIL ENGINE AMERICAN SOCIETY OF TESTING AND MATERIALS AMERICAN WELDING SOCIETY INTERNATIONAL BUILDING CODE INTERNATIONAL CODE COUNCIL

OCCUPATIONAL SAFETY AND

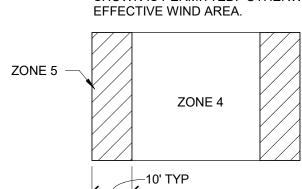
HEALTH ASSOCIATION

GENERAL SYMBOLS	GENERAL SYMBOLS			
CONCRETE (CAST IN PLACE OR PRECAST)		1		
CONCRETE BLOCK (CMU)	EXTERIOR ELEV	A1	REVISION NOTE	1
EARTH	INTERIOR ELEV	1	PLAN NOTE (KEY NOTE)	
FACE BRICK		A1	,	
FREE DRAINING FILL OR GRAVEL (DESIGNATE ON DRAWINGS)	BUILDING SECTIONS	1 A1	DEMOLITION NOTE	A
PLYWOOD				
RIGID INSULATION	WALL SECTIONS	1 A1	COLUMN GRID (LETTERS VERT. NUMBERS HORIZ)	A
ROUGH LUMBER	DETAILS	1		
STRUCTURAL STEEL		A1		



BUILDING WALL PRESSURES		10 S.F.	100 S.F.	500 S.F.
NEGATIVE	ZONE 4	-26 PSF	-22 PSF	-20 PSF
PRESSURES	ZONE 5 (h <u>&lt;</u> 60')	-31 PSF	-25 PSF	-20 PSF
POSITIVE PRESSURES	ZONES 4 & 5	+24 PSF	+20 PSF	+18 PSF
ROOF PRESSURES		10 S.F.	50 S.F.	100 S.F.
	ZONE 1	-41 PSF	-35 PSF	-32 PSF
NEGATIVE PRESSURES	ZONE 2	-54 PSF	-46 PSF	-42 PSF
	ZONE 3	-73 PSF	-57 PSF	-50 PSF
POSITIVE PRESSURES	ALL ZONES	+16 PSF	+16 PSF	+16 PSF

#### INTERPOLATION FOR EFFECTIVE WIND AREAS BETWEEN THOSE SHOWN IS PERMITTED. OTHERWISE USE PRESSURE FOR SMALLER



WALL ELEVATION

ZONE 1
ZONE 2 ZONE 3
ROOF PLAN

### A. BUILDING CODES:

- 1. GOVERNING BUILDING CODE: 2018 INTERNATIONAL BUILDING CODE. 2. REFERENCED CODES: a. ASCE 7-16 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.
- RISK CATEGORY: II 4. EXPOSURE CATEGORY: C

#### B. DEAD LOADS: 1. ROOF UNIFORM DEAD LOAD: 20 PSF

#### C. LIVE LOADS: 1. UNIFORM ROOF LIVE: 20 PSF

### D. SNOW LOADS:

- 1. GROUND SNOW LOAD: 25 PSF
- 2. SNOW IMPORTANCE FACTOR, I<sub>s</sub>: 1.0, Category II 3. SNOW EXPOSURE FACTOR, C<sub>e</sub>: 1.0
- 4. THERMAL FACTOR, Ct. 1.2, UNHEATED OR OPEN AIR (WORST CASE FOR OVERHANG) 5. FLAT-ROOF SNOW LOAD, pf: 21.0 PSF 6. ROOF SLOPE FACTOR, Cs. 1.0
- 7. SLOPED-ROOF SNOW LOAD, Ps: 21.0 PSF 8. DRIFT LOAD AT VESTIBULE: SEE DETAIL 16 THIS SHEET.
- E. WIND LOADS:
- 1. ULTIMATE (BASIC) WIND SPEED (3 SECOND GUST), V: 108 MPH 2. ALLOWABLE STRESS DESIGN (ASD) WIND SPEED, Vasd: 83.7 MPH
- 3. TOPOGRAPHIC FACTOR, Kzt. 1.0
- 4. DIRECTIONALITY FACTOR, Kd: 0.85 5. GUST EFFECT FACTOR: 0.85
- 6. INTERNAL PRESSURE COEFFICIENT: ±0.18, ENCLOSED 7. COMPONENTS AND CLADDING GROSS, ULTIMATE PRESSURES: SEE TABLE THIS SHEET. POSITIVE PRESSURES ACT TOWARD THE STRUCTURE, NEGATIVE PRESSURES ACT AWAY FROM THE STRUCTURE. MULTIPLY TABULATED VALUES BY 0.6 TO CONVERT TO SERVICE-LEVEL PRESSURES.
- F. SEISMIC LOADS: 1. SEISMIC IMPORTANCE FACTOR, I<sub>E</sub>: 1.00, CATEGORY II
- 3. SPECTRAL RESPONSE COEFFICIENTS. a. S<sub>s</sub>: 0.097 b. S<sub>1</sub>: 0.062

2. SITE CLASS: D

- c. S<sub>DS</sub>: 0.103 d. S<sub>D1</sub>: 0.099
- 4. SEISMIC DESIGN CATEGORY: B 5. BASIC SEISMIC FORCE-RESISTING SYSTEM: STEEL ORDINARY CANTILEVER COLUMN SYSTEM
- 6. DESIGN BASE SHEAR: 500 LBS . SEISMIC RESPONSE COEFFICIENTS(S), Cs: 0.034
- 8. RESPONSE MODIFICATION COEFFICIENT(S), R: 1 1/4 9. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

#### G. SOIL LOADS: 1. DESIGN COEFFICIENT OF FRICTION: 0.35

- 2. ASSUMED ALLOWABLE SOIL BEARING PRESSURE: 1500 PSF 3. BOTTOM OF EXTERIOR FOOTINGS SHALL EXTEND A MINIMUM OF 4'-0" BELOW FINAL GRADE (6'-0" BELOW FINAL GRADE IN UNHEATED STRUCTURES).
- 4. OWNER IS RESPONSIBLE FOR VERIFICATION OF SOIL CONDITIONS FOR CAPABILITY OF SUPPORTING 1500 PSF SOIL BEARING PRESSURE.
- H. ROOF RAIN LOAD DATA: 1. RAIN INTENSITY, I, 15 MIN: 6.95 IN/HR

### 2. RAIN INTENSITY, I, 60 MIN: 3.33 IN/HR

#### J. MISCELLANEOUS:

- 1. STRUCTURAL DRAWINGS ARE INTENDED TO BE USED WITH DRAWINGS FOR ALL OTHER DISCIPLINES. CONTRACTOR IS RESPONSIBLE FOR COORDINATING REQUIREMENTS INTO THEIR SHOP DRAWINGS AND
- 2. NO OPENING SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE
- STRUCTURAL ENGINEER. 3. NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBER SHALL BE MADE WITHOUT THE WRITTEN
- APPROVAL OF THE STRUCTURAL ENGINEER.
- 4. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON
- THE STRUCTURAL FRAMING, INCLUDING SLABS ON GRADE. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
- 5. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS
- THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES. 6. DO NOT SCALE THESE DRAWINGS. USE DIMENSIONS SHOWN.
- 7. THE CONTRACTOR SHALL INFORM THE ENGINEER IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY FOR SUCH DEVIATION BY THE ENGINEER'S APPROVAL OF SHOP DRAWINGS, PRODUCT DATA, ETC., UNLESS THE ARCHITECT HAS BEEN INFORMED OF SUCH DEVIATION AT THE TIME OF SUBMISSION AND THE ARCHITECT HAS GIVEN APPROVAL TO THE SPECIFIC DEVIATION.
- 8. ALL THINGS, WHICH IN THE OPINION OF THE CONTRACTOR, APPEAR TO BE DEFICIENCIES, OMISSIONS, CONTRADICTIONS, OR AMBIGUITIES IN THE PLANS AND SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER. PLANS AND/OR SPECIFICATIONS WILL BE CORRECTED, OR WRITTEN
- INTERPRETATION OF THE ALLEGED DEFICIENCY, OMISSION, CONTRADICTION OR AMBIGUITY WILL BE MADE BY THE ARCHITECT BEFORE THE AFFECTED WORK PROCEEDS.
- 9. ALL DIMENSIONS SHALL BE CHECKED AGAINST REQUIREMENTS OF OTHER CONTRACT DOCUMENTS. FIELD VERIFY DIMENSIONS RELATING TO EXISTING CONDITIONS PRIOR TO ORDERING MATERIALS AND
- FABRICATION. 10. WHERE DIMENSION OR WEIGHTS OF EQUIPMENT OR SYSTEMS ARE VARIABLE FROM MANUFACTURER TO MANUFACTURER, VERIFY DIMENSIONS AND WEIGHTS SHOWN ON DRAWINGS WITH SELECTED
- MANUFACTURER PRIOR TO ORDERING MATERIALS. NOTIFY STRUCTURAL ENGINEER OF DISCREPANCIES. DO NOT PLACE EQUIPMENT WHEN SHIPPING OR OPERATING WEIGHT EXCEEDS WEIGHT INDICATED ON STRUCTURAL DRAWINGS.

# IN THE STATE WHERE THE PROJECT IS LOCATED AND EMPLOYED BY THE CONTRACTOR.

11. NO MODIFICATION, ALTERATION OR REPAIR SHALL BE MADE WITHOUT PRIOR REVIEW BY THE STRUCTURAL ENGINEER. SUBMIT DETAILS AND CALCULATIONS PREPARED BY A PROFESSIONALENGINEER, REGISTERED

### POST-INSTALLED ANCHOR EMBEDMENT REQUIREMENTS

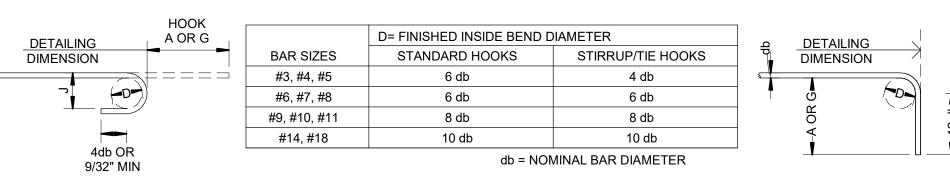
EXPANSION ANCHOR EMBEDMENTS			SLEEVE ANCHOR EMBEDMENTS		HILTI HIT-HY 200 MAX ADHESIVE ANCHORING SYSTEM W/ REBAR				HILTI HIT-HY 200 MAX ADHESIVE ANCHORING SYSTEM W/ HAS ROI						
ANCHOR SIZE	MIN. EMBED	STD. EMBED	MAX. EMBED		ANCHOR SIZE	STD. EMBED		BAR SIZE	STD. EMBED	MAX. EMBED		ROD DIA	MIN. EMBED	STD. EMBED	MAX EMB
3/8" Ø	1 5/8"	2 1/2"	N/A		3/8" Ø	1 1/4"		#3	3 3/8"	7 1/2"	1 [	1/2"	2 3/4"	4 1/2"	10
1/2" Ø	2 1/4"	3 1/2"	4 3/4"		1/2" Ø	1 1/2"		#4	4 1/2"	10"		5/8"	3 1/8"	5 5/8"	12 1
5/8" Ø	2 3/4"	4"	5 1/2"		5/8" Ø	2"		#5	5 5/8"	12 1/2"		3/4"	3 1/2"	6 3/4"	15
3/4" Ø	3 1/4"	4 3/4"	6 1/2"	RFI	FERENCE P	RODUCT =	:						*		

REFERENCE PRODUCT =  $\frac{3}{4}$  REFERENCE PRODUCT =  $\frac{3}{4}$  REFERENCE PRODUCT =  $\frac{3}{4}$  HILTI HLC SLEEVE ANCHORS.

EMBEDMENT DEPTHS INDICATED IN DETAILS GOVERN.

POST-INSTALLED ANCHOR PRODUCT SUBSTITUTIONS SHALL BE APPROVED BY ENGINEER PRIOR TO USE.

### STANDARD HOOK DETAILS

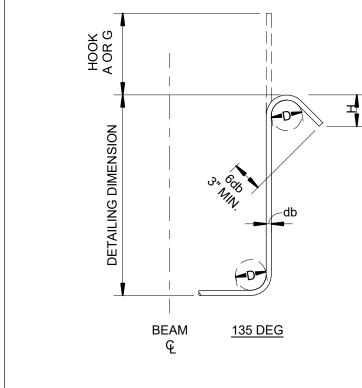


180 DEG.

0.75		IONS OF STANDARD 1 HOOKS, ALL GRADES		DIMENSIONS OF STANDARD 90- DEG HOOKS, ALL GRADES		
	A OR G	J	D	A OR G	D	
#3	5"	3"	2 1/4"	6"	2 1/4"	
#4	6"	4"	3"	8"	3"	
#5	7"	5"	3 3/4"	10"	3 3/4"	
#6	8"	6"	4 1/2"	1'-0"	4 1/2"	
#7	10"	7"	5 1/4"	1'-2"	5 1/4"	
#8	11"	8"	6"	1'-4"	6"	
#9	1'-3"	11 3/4"	9 1/2"	1'-7"	9 1/2"	
#10	1'-5"	1'-1 1/4"	10 3/4"	1'-10"	10 3/4"	
#11	1'-7"	1'-2 3/4"	12"	2'-0"	12"	
#14	2'-3"	1'- 9 3/4"	18 1/4"	2'-7"	18 1/4"	
#18	3'-0"	2'-4 1/2"	24"	3'-5"	24"	

D= BEND DIAMETER

FOR #3, #4, #5- 6db FOR #6, #7, #8- 12db A OR G 135 DEG BEAM BEAM



<u>90 DEG.</u>

STIRRUP HOOKS	

		(TIE BENDS		
BAR		90 DEG	135	DEG
SIZE	D	A OR G	A OR G	APPROX. H
#3	1 1/2"	4"	4"	2 1/2"
#4	2"	4 1/4"	4 1/2"	3"
#5	2 1/2"	6"	5 1/2"	3 3/4"
#6	4 1/2"	1'-0"	7 3/4"	4 1/2"
#7	5 1/4"	1'-2"	9"	5 1/4"
#8	6"	1'-4"	10 1/4"	6"

SEISMIC STIRRUP/ TIE

	135 D	EG SEISMIC HO	ЭK
SIZE	D	A OR G	APPROX. H
#3	1 1/2"	4 1/4"	3"
#4	2"	4 1/2"	3"
#5	2 1/2"	5 1/2"	3 3/4"
#6	4 1/2"	7 3/4"	4 1/2"
#7	5 1/4"	9"	5 1/4"
#8	6"	10 1/4"	6"

#### REINFORCING STEEL LAPS MAINTAIN CONTINUITY OF REINFORCING WITH STAGGERED LAP SPLICES. REFER TO THE TYPICAL CORNER REINFORCING, WALL AND FOOTING STEP DETAILS. REINFORCING LAP SPLICES SHALL BE IN ACCORDANCE WITH THE CHART BELOW. SPLICE LENGTHS ARE SPECIFIED IN INCHES.

CONCRETE	SPLICE	#4	#5	#6	#7	#8	#9
STRENGTH	TYPE						
3000 PSI	CLASS A TOP	29	36	43	63	72	81
	CLASS A	22	28	33	48	55	62
	CLASS B TOP	38	47	56	81	93	105
	CLASS B	29	36	43	63	72	81
4000 PSI	CLASS A TOP	25	31	37	54	62	70
	CLASS A	19	24	29	42	48	54
	CLASS B TOP	33	41	49	71	81	91
	CLASS B	25	31	37	54	62	70
4500 PSI	CLASS A TOP	24	30	35	51	59	66
	CLASS A	18	23	27	40	45	51
	CLASS B TOP	31	38	46	67	76	86
	CLASS B	24	30	35	51	59	66
5000 PSI	CLASS A TOP	23	28	34	49	56	63
	CLASS A	17	17	17	38	43	48
	CLASS B TOP	29	36	44	63	72	81
	CLASS B	23	28	34	49	56	63

1. CLASS A LAP: USE ONLY WHERE SPECIFIED.

- 2. CLASS B LAP: USE UNLESS NOTED OTHERWISE. 3. TOP BARS: HORZIONTAL REINFORCEMENT WITH MORE THAN 12 IN OF FRESH CONCRETE IS PLACED BELOW EXCEPT
- WALL REINFORCEMENT. 4. TABULATED VALUES ARE BASED UPON A MINIMUM REINFORCING BAR YIELD STRENGTH OF 60,000 PSI AND NORMAL
- WEIGHT CONCRETE. 5. CLEAR SPACING BETWEEN BARS AND CLEAR COVER MUST BE EQUAL TO OR GREATER THAN TWO BAR DIAMETERS.
- 6. THESE SPLICE LENGTHS HAVE BEEN DEVELOPED IN ACCORDANCE WITH ACI 318. 7. THIS TABLE DOES NOT APPLY TO EPOXY-COATED REBAR.

8.	FOR GRADE 80 REINFORCING BARS MULTIPLY TABULATED VALUES BY 1.53.	
9.	FOR LIGHTWEIGHT CONCRETE MULTIPLY TABULATED VALUES BY 1.33.	

MINIMUM CONCRETE COVER F	PER ACI318
CAST-IN-PLACE CONCRETE	MIN COVER, IN.
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3
B. CONCRETE EXPOSED TO EARTH OR WEATHER:	
NO. 6 - NO. 18 BAR	2
NO. 5 BAR, W31 OR D31 WIRE, AND SMALLER	1 1/2
C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS, JOISTS: NO. 14 AND NO. 18 BARS NO. 11 AND SMALLER	1 1/2 3/4
BEAMS, COLUMNS: PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1 1/2

Client Name CLINTON COUNTY

800 556-4491

© Origin Design Co.

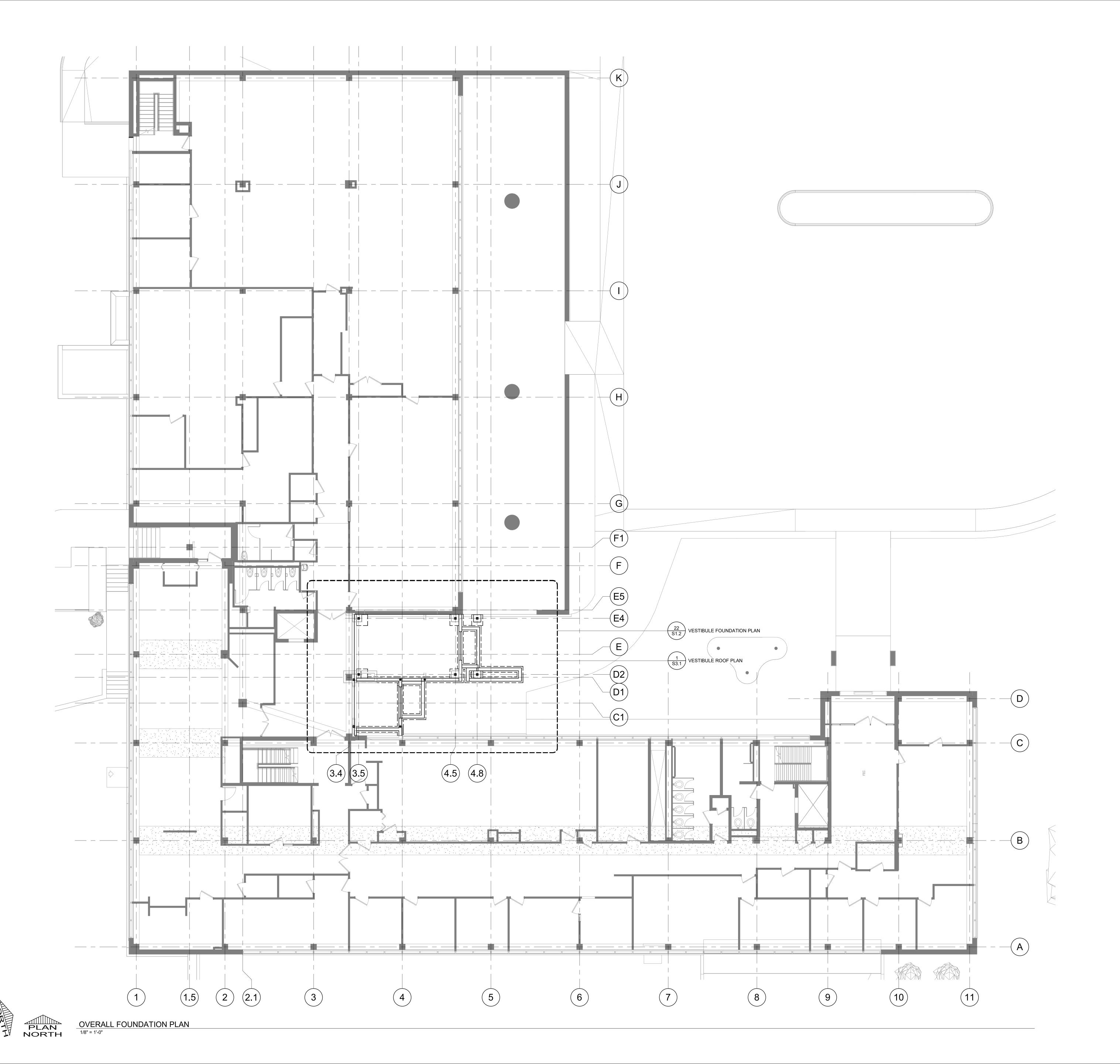
Project Name **ADMINISTRATION BUILDING** -

Location / Description 1900 N 3RD ST. CLINTON, IA 52732

ALTERATIONS

Sheet Title

**ABBREVIATIONS AND STRUCTURAL NOTES** 



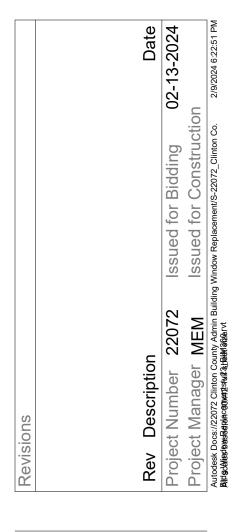


Client Name
CLINTON COUNTY

Project Name

ADMINISTRATION
BUILDING ADDITION &
ALTERATIONS

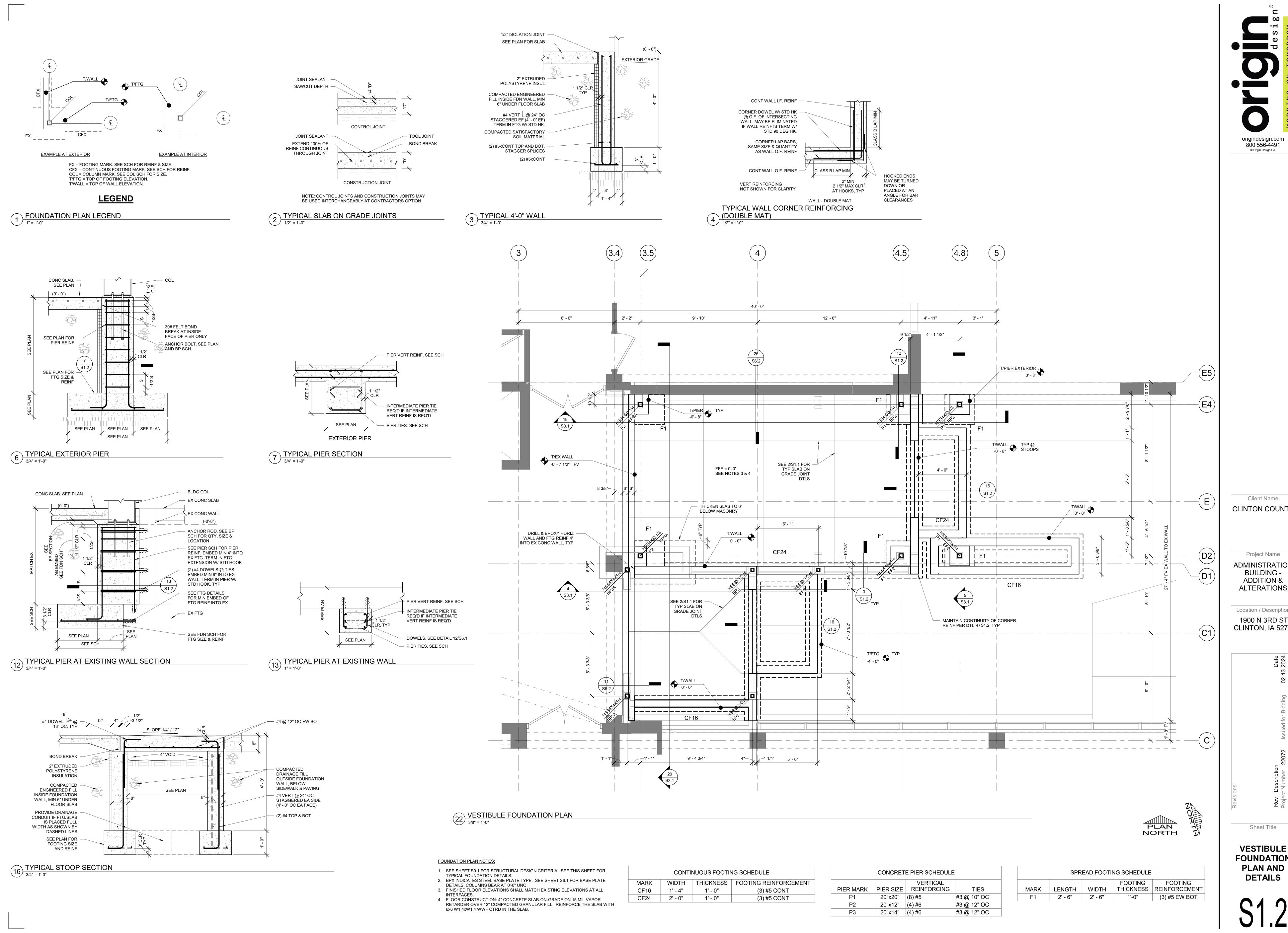
Location / Description 1900 N 3RD ST. CLINTON, IA 52732



Sheet Title

OVERALL FOUNDATION PLAN

S1.1





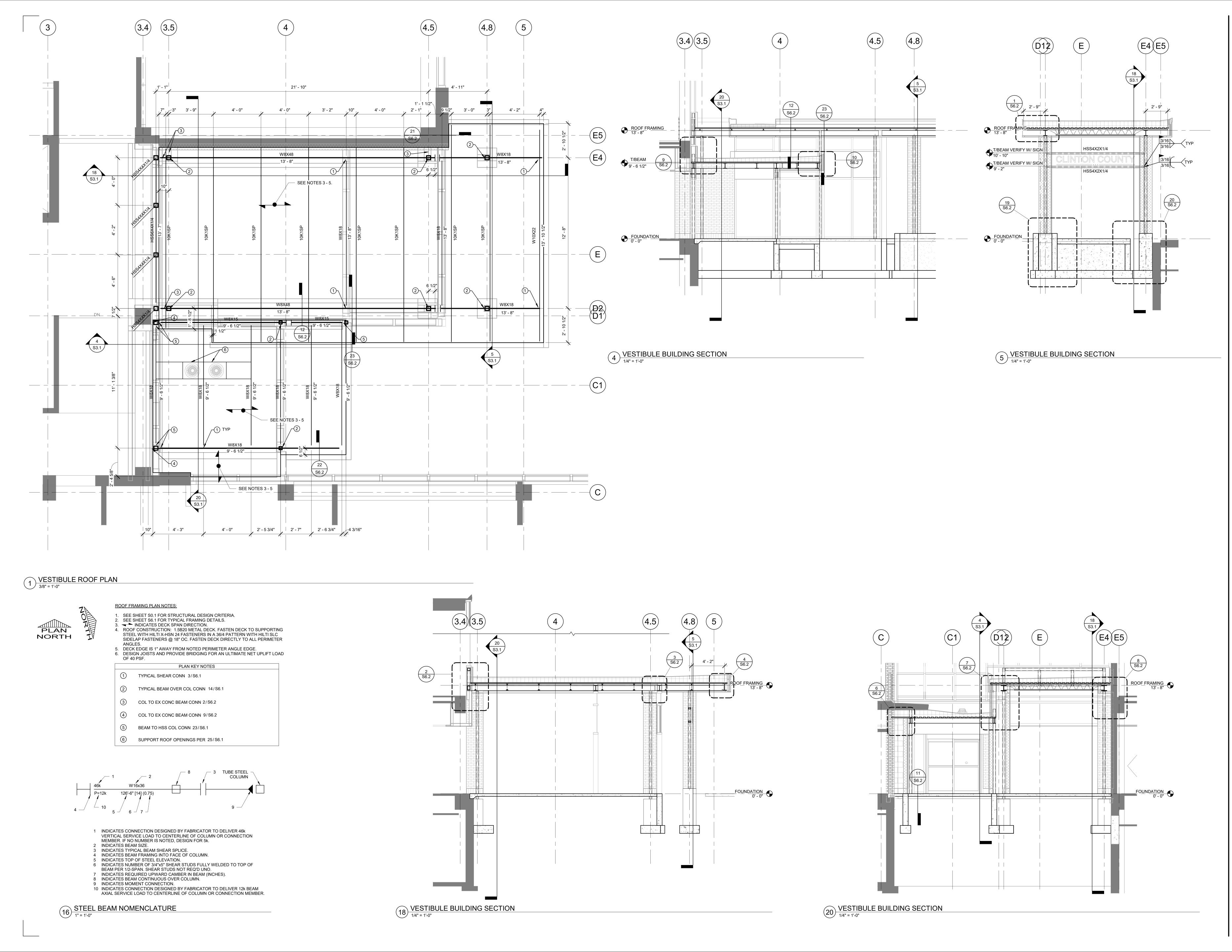
Client Name **CLINTON COUNTY** 

Project Name **ADMINISTRATION BUILDING** -

Location / Description 1900 N 3RD ST. CLINTON, IA 52732

Sheet Title

**VESTIBULE FOUNDATION PLAN AND DETAILS** 



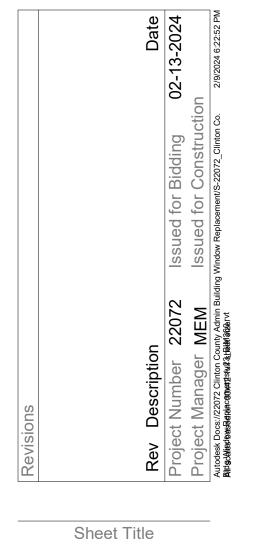


Client Name
CLINTON COUNTY

Project Name

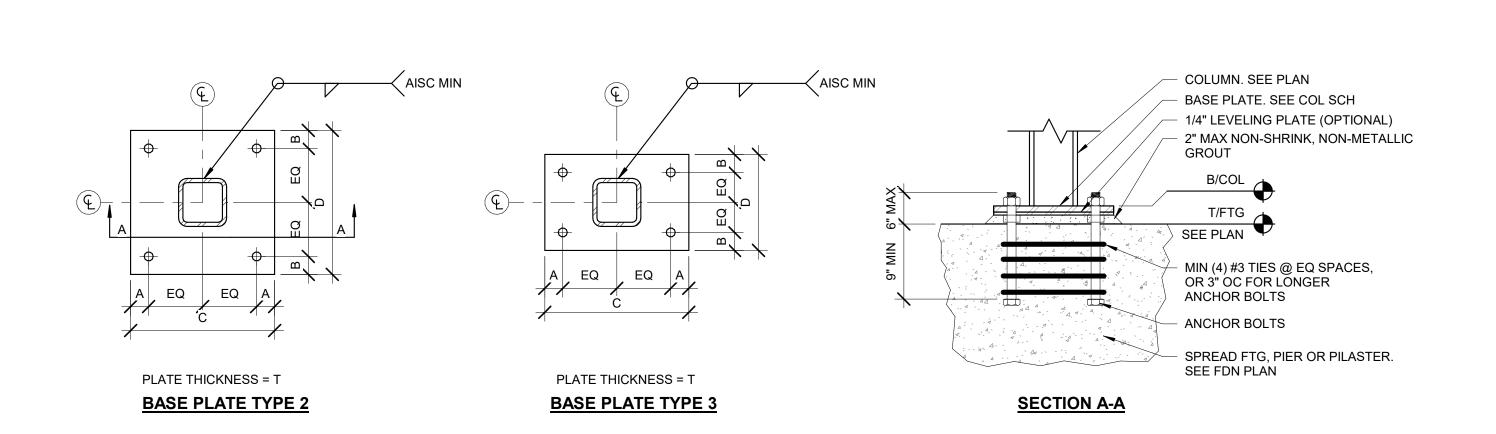
ADMINISTRATION
BUILDING ADDITION &
ALTERATIONS

Location / Description 1900 N 3RD ST. CLINTON, IA 52732



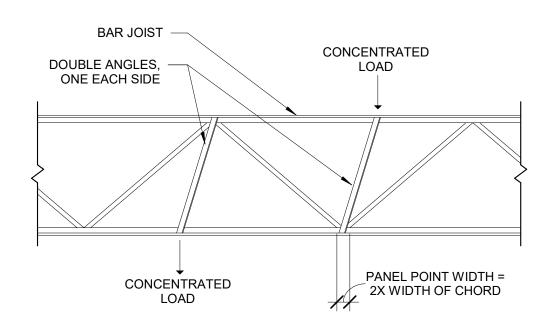
VESTIBULE ROOF PLAN AND SECTIONS

S3.1



MARK	"A"	"B"	"C"	"D"	THICKNESS	ANCHORS	NOTES
BP2	1 1/2"	1 1/2"	10"	10"	3/4"	(4) 3/4" ANCHOR BOLTS	6" MIN EMBED, OPTIONAL POST- INSTALL W/ HILTI HY-200
BP3	1"	1"	7"	6"	1/2"	(4) 1/2" KH-EZ	4" MIN EMBED
врза	1 1/2"	1 1/2"	10"	9"	3/4"	(4) 1/2" KH-EZ	4" MIN EMBED

## 1 BASEPLATES 1 1/2" = 1'-0"

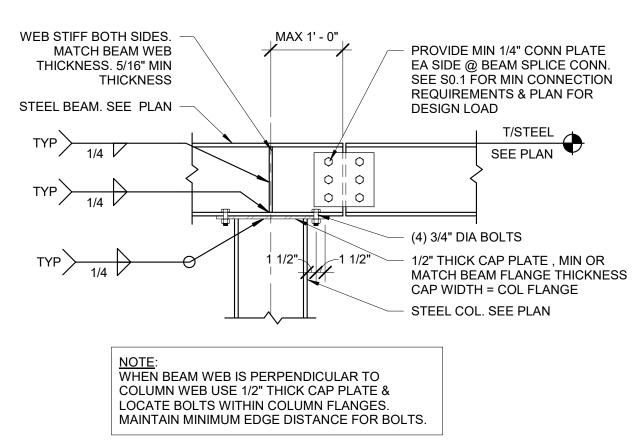


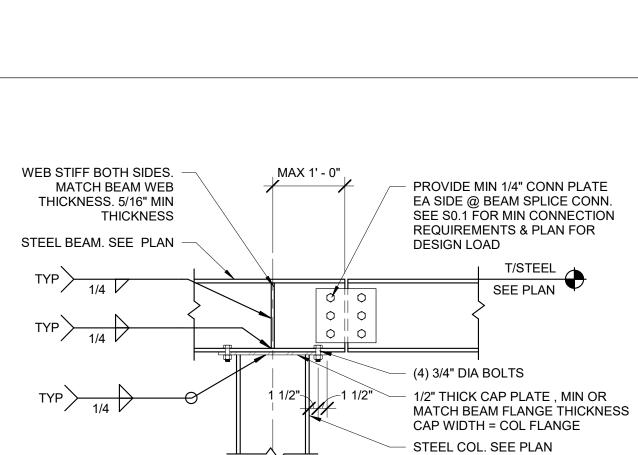
NOTE:

1. JOIST CHORDS ARE NOT DESIGNED FOR CONCENTRATED LOADS. PLACE LOADS AT PANEL POINTS OR WELD ADDITIONAL DOUBLE ANGLE MEMBERS, ONE EACH SIDE, FROM POINT OF CONCENTRATED LOAD TO NEAREST PANEL POINT ON OPPOSITE CHORD OF THE JOIST. ADDITIONAL ANGLES SHALL BE SUPPLIED AND INSTALLED BY GENERAL CONTRACTOR. PROVIDE L1X1X1/8. WELD IN PLACE. (ALL AROUND 1/8" FILLET WELD)

TYPICAL WEB MEMBER AT BAR JOIST CONCENTRATED LOAD DETAIL

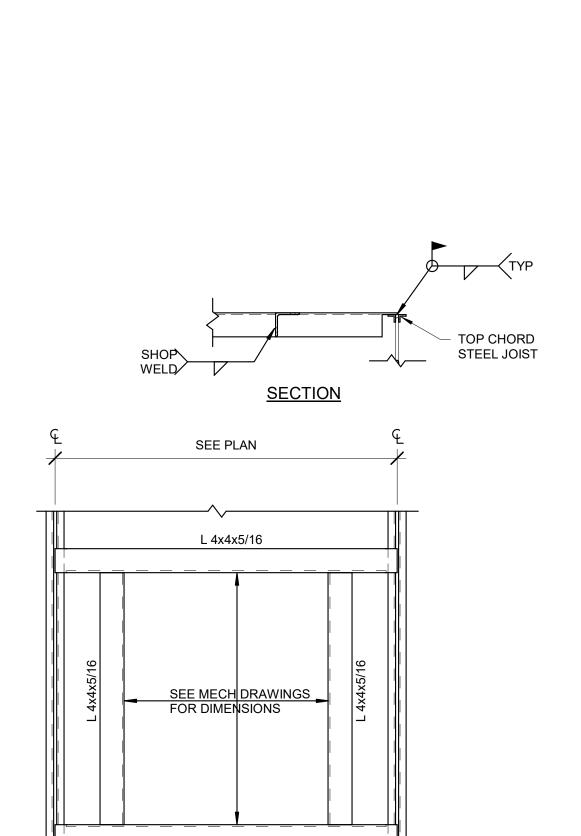
3/4" = 1'-0"





TYPICAL BEAM-OVER-COLUMN SECTION

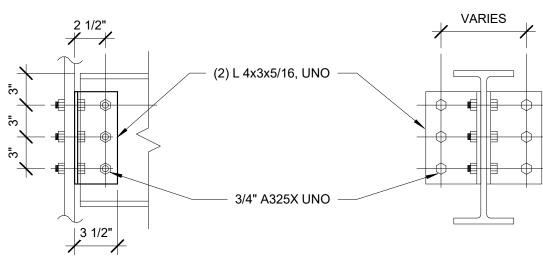
3/4" = 1'-0"



 PROVIDE RECESSED SUMP PANS AT EA ROOF DRAIN. TYPICAL DETAIL OF ROOF VENT/ROOF DRAIN

L 4x4x5/16

SHOP-FABRICATED FRAME

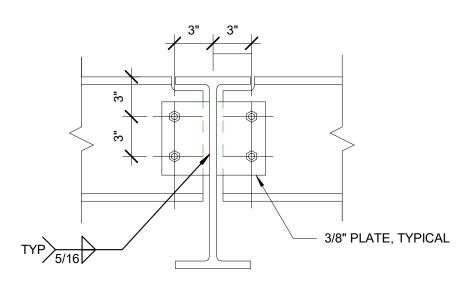


DOUBLE ANGLE CONNECTION					
NOMINAL BEAM DEPTH, INCHES	ROWS OF BOLTS	LENGTH OF ANGLE	REMARKS		
W36	7	1'-8 1/2"	SEE SECTION B-B FOR BACK TO BACK BEAM CONN USE 1" DIA A325x, USE(2) L5x5x3/8		
W30-W33	7	1'-8 1/2"	SEE SECTION B-B FOR BACK TO BACK BEAM CONN		
W24-W27	5	1'-2 1/2"	SEE SECTION B-B FOR BACK TO BACK BEAM CONN		
W16-W21	4	11 1/2"	SEE SECTION B-B FOR BACK TO BACK BEAM CONN		
W14	3	8 1/2"	SEE SECTION B-B FOR BACK TO BACK BEAM CONN		
W12	2	5 1/2"	SEE SECTION B-B FOR BACK TO BACK BEAM CONN		
W8-W10	-	-	SEE SECTION A-A		

1. NUMBER OF ROWS IS EQUAL TO NUMBER 0F BOLTS TO ENCLOSED WEB. 2. ALL FRAMING CONNECTIONS SHALL CONFORM TO SCHEDULE UNLESS DETAILED OTHERWISE ON FRAMING PLANS.

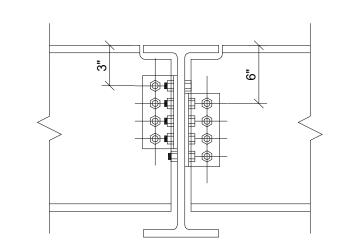
FOR MIS-ALIGNED BOLT HOLES, PROVIDE FIELD WELDS. NOTIFY A/E OF LOCATIONS USING FIELD WELDED CONNECTION. 4. THE STEEL FABRICATOR IS TO DESIGN THE CONNECTIONS FOR THE REACTIONS INDICATED ON THE PLANS. THIS TABLE DEPICTS MINIMUM CONNECTION REQUIREMENTS.

MINIMUM BOLTED CONNECTION 3 REQUIREMENTS
1/8" = 1'-0"

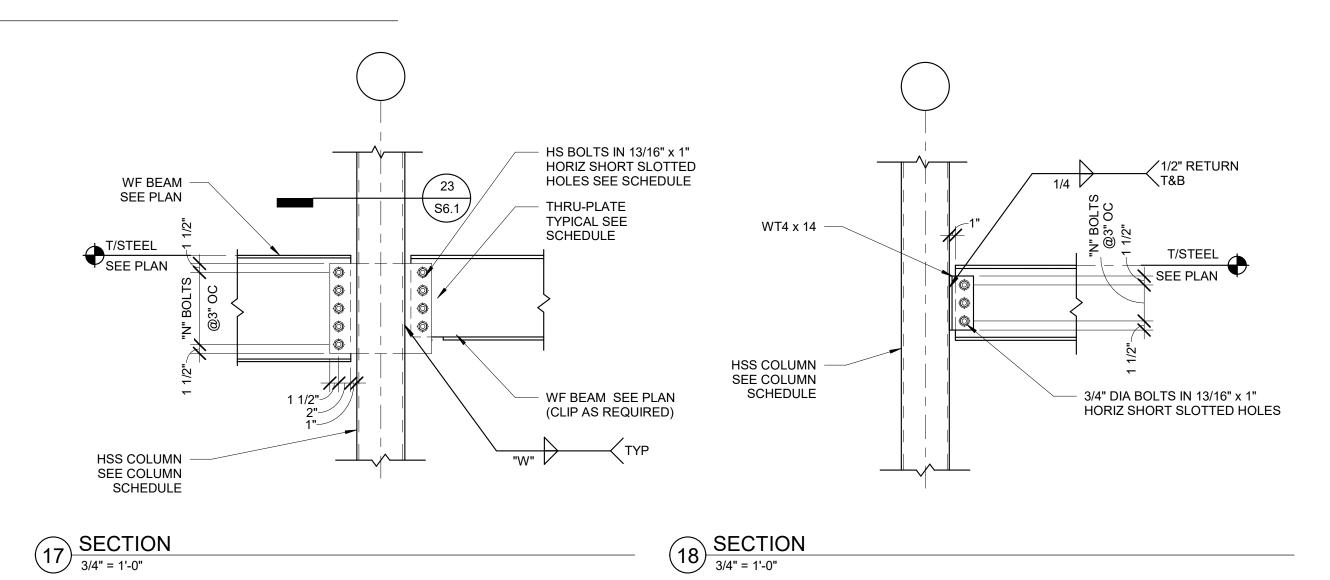


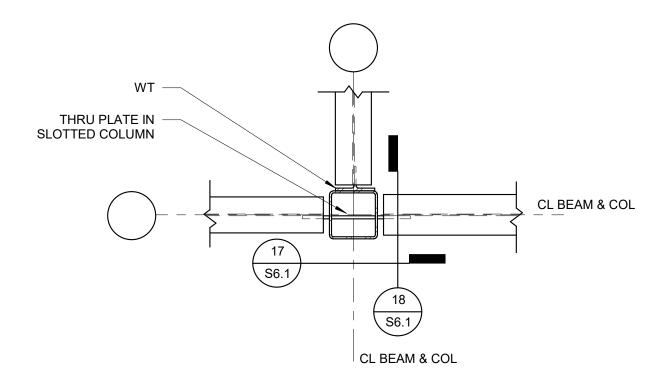
NOMINAL BEAM ROWS OF LENGTH OF REMARKS DEPTH, INCHES | BOLTS | PLATE 8-10 2 6"

SINGLE-SHEAR PLATE CONNECTION, SECTION A-A (BEAMS 10" AND LESS)



DOUBLE ANGLE CONNECTION @ BACK TO BACK BEAMS (BEAMS 12" AND GREATER)





CONNECTION SCHEDULE					
BEAM SIZE	"N" NO.OF 3/4" DIA BOLTS	"T" PLATE THICKNESS	"W" WELD SIZE		
W8, W10	2	1/4"	3/16"		
W12, W14	3	1/4"	3/16"		
W16	4	1/4"	3/16"		
W18, W21	5	5/16"	1/4"		
W24	6	5/16"	1/4"		
W27	7	5/16"	1/4"		
W30	8	3/8"	5/16"		
W33	9	7/16"	3/8"		
W36	10	7/16"	3/8"		

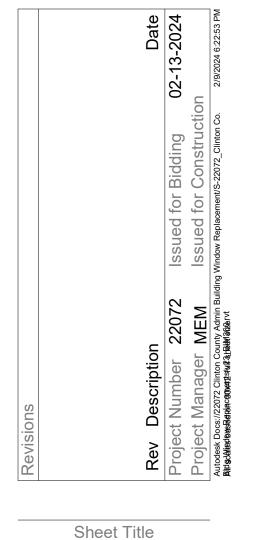
WF BEAM TO HSS COLUMN CONNECTION 23 DETAIL
3/4" = 1'-0"

Client Name **CLINTON COUNTY** 

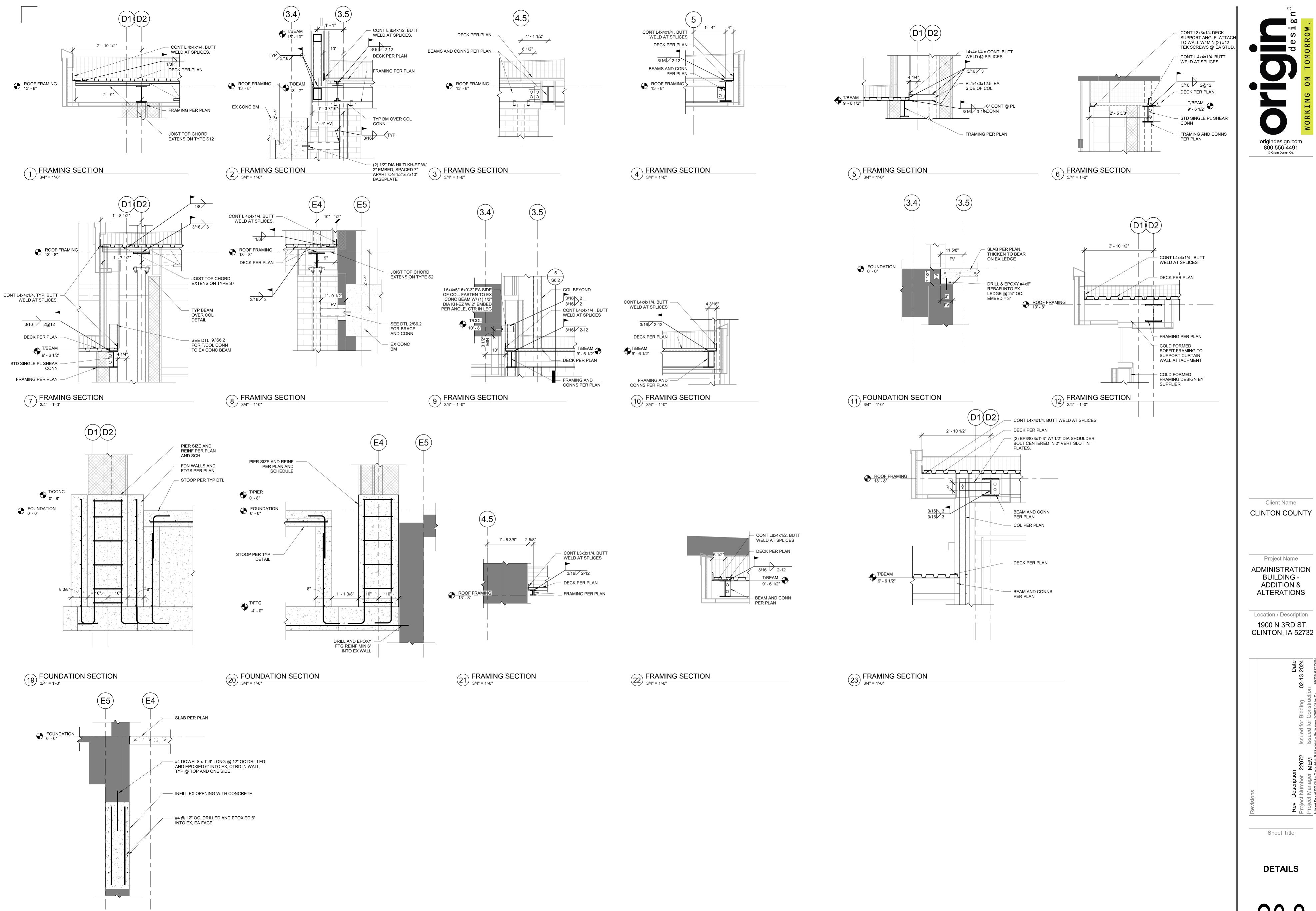
800 556-4491 © Origin Design Co.

Project Name **ADMINISTRATION BUILDING** -**ADDITION & ALTERATIONS** 

Location / Description 1900 N 3RD ST. CLINTON, IA 52732



TYPICAL STEEL **DETAILS** 



FOUNDATION SECTION

3/4" = 1'-0"